

Application for Preliminary Plat

Municipality of Anchorage
Planning Department
PO Box 196650
Anchorage, AK 99519-6650



PETITIONER*		PETITIONER REPRESENTATIVE (IF ANY)	
Name (last name first)	James Sears, Southcentral Foundation	Name (last name first)	Dave Hale, R&M Consultants, Inc.
Mailing Address	4501 Diplomacy Drive	Mailing Address	9101 Vanguard Dr.
	Anchorage, AK. 99508		Anchorage, AK. 99507
Contact Phone – Day	Evening	Contact Phone – Day	Evening
907-729-5261		907-646-9651	
Fax		Fax	
E-mail	jsears@southcentralfoundation.com	E-mail	dhale@rmconsult.com

*Report additional petitioners or disclose other co-owners on supplemental form. Failure to divulge other beneficial interest owners may delay processing of this application.

PROPERTY INFORMATION			
Property Tax # (000-000-00-000): 008-024-09-000; 008-024-06-000; 008-024-05-000			
Site Street Address: 4330 Elmore Road			
Current legal description: (use additional sheet if necessary)			
Tract G5 & G6, Athenian Village Subdivision, Plat No. 83-62 Tract G4-1, Athenian Village Subdivision, Plat No. 2007-114			
Zoning: PLI/B-3	Acreage: 2.56	Underlying Plat #: 83-62 & 2007-114	Grid #: SW 1735
# Lots:	# Tracts: 3	Total # parcels: 3	

PROPOSED SUBDIVISION INFORMATION		
Proposed legal description: (use additional sheet if necessary)		
Tract G5-1, Athenian Village Subdivision		
# Lots: None	# Tracts: 1	Total # parcels: 1

I hereby certify that (I am)/(I have been authorized to act for) owner of the property described above and that I petition to subdivide it in conformance with Title 21 of the Anchorage Municipal Code of Ordinances. I understand that payment of the application fee is nonrefundable and is to cover the costs associated with processing this application, and that it does not assure approval of the subdivision. I also understand that assigned hearing dates are tentative and may have to be postponed by Planning Department staff or the Platting Authority for administrative reasons.

Signature

☐ Owner

☒ Representative

(Representatives must provide written proof of authorization)

8/14/19
Date

Print Name

Dave Hale

Accepted by:	Poster & Affidavit:	Fee:	Case Number:	Requested Meeting Date:
Karlie Greeng	N/A	\$3,775	S12514	P.O. 10/7/19

COMPREHENSIVE PLAN INFORMATIONAnchorage 2020 Urban/Rural Services: ☒ Urban ☐ Rural

Anchorage 2020 Major Elements – site is within or abuts:

- ☒ Major employment center ☒ Redevelopment/mixed use area ☒ Town center
☐ Neighborhood commercial center ☐ Industrial reserve
☐ Transit - supportive development corridor ☐ District/area plan area: _____

Chugiak-Eagle River Land Use Classification:

- ☐ Commercial ☐ Industrial ☐ Parks/opens space ☐ Public lands/institutions ☐ Town center
☐ Transportation/community facility ☐ Alpine/slope affected ☐ Special study area ☐ Development Reserve
☐ Residential at _____ dwelling units per acre ☐ Environmentally sensitive area

Girdwood- Turnagain Arm Land Use Classification

- ☐ Commercial ☐ Industrial ☐ Parks/opens space ☐ Public lands/institutions ☐ Resort
☐ Transportation/community facility ☐ Alpine/slope affected ☐ Special study area ☐ Reserve
☐ Residential at _____ dwelling units per acre ☐ Mixed use ☐ Rural homestead

ENVIRONMENTAL INFORMATION (All or portion of site affected)

- Wetland Classification: ☒ None ☐ "C" ☐ "B" ☐ "A"
 Avalanche Zone: ☒ None ☐ Blue Zone ☐ Red Zone
 Floodplain: ☒ None ☐ 100 year ☐ 500 year
 Seismic Zone (Harding/Lawson): ☐ "1" ☒ "2" ☐ "3" ☐ "4" ☐ "5"

RECENT REGULATORY INFORMATION (Events that have occurred in last 5 years for all or portion of site)

- ☒ Rezoning - Case Number: Rezone will be concurrent with platting action
☐ Preliminary Plat ☐ Final Plat - Case Number(s): _____
☐ Conditional Use - Case Number(s): _____
☐ Zoning variance - Case Number(s): _____
☐ Land Use Enforcement Action for _____
☐ Building or Land Use Permit for _____
☐ Wetland permit: ☐ Army Corp of Engineers ☐ Municipality of Anchorage

POTABLE WATER AND WASTE WATER DISPOSAL

- Potable Water provided by: ☒ Public utility ☐ Community well ☐ Private well
 Wastewater disposal method: ☒ Public utility ☐ Community system ☐ Private on-site

APPLICATION CHECKLIST (Only one copy of applicable items is required for initial submittal)

Fee:

- Plat: Copies ☒ Plat, full size ☒ 8½x11 reduced copy ☒ Watershed sign off form, completed
 Other ☐ Aerial photo ☐ Housing stock ☒ Zoning ☐ One copy, original application
 (35 sets short plat; 45 sets long plat)

Property Title: ☐ Certificate to Plat

Documents to provided unless waived by Platting Officer:

- ☒ Site topography (4 copies minimum) Waived by _____
☒ Soils investigation and analysis reports (4 copies minimum) Waived by _____
☒ Subdivision drainage plan Waived by _____

Application for Right-of-Way and Easement Vacation

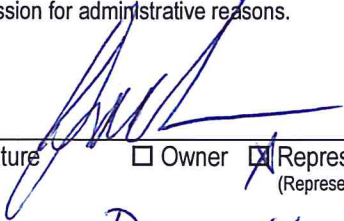
Municipality of Anchorage
Planning Department
PO Box 196650
Anchorage, AK 99519-6650

PETITIONER*		PETITIONER REPRESENTATIVE (if any)	
Name (last name first)	James Sears, SCF	Name (last name first)	Dave Hale, PLS
Mailing Address	4501 Diplomacy Dr.	Mailing Address	9101 Vanguard Dr.
	Anchorage, AK. 99508		Anchorage, AK. 99507
Contact Phone – Day:	(907) 729-5261	Contact Phone – Day:	(907) 646-9651
Evening:		Evening:	
Fax:		Fax:	
E-mail:	jsears@southcentralfoundation.com	E-mail:	dhale@rmconsult.com

*Report additional petitioners or disclose other co-owners on supplemental form. Failure to divulge other beneficial interest owners may delay processing of this application.

RIGHT-OF-WAY AND/OR INFORMATION		
Benefiting Property Tax # (000-000-00-000): 008-024-09-000; 008-024-06-000; 008-024-05-000		
Site Street Address: 4330 Elmore Road		
Description of right-of-way/easement: (use additional sheet if necessary)		
30' Telephone, Electric, & Sanitary Sewer Easement That straddles a portion of Tract G4, G5, and G6, Athenian Village Subd, Plat 83-62. Request vacation of easement to accommodate new building location. All utilities within current easement will be relocated during construction to a dedicated easement to the north.		
Zoning: PLI, B-3	Acreage: 3,599 Sq.Ft. (0.083 ac.)	Grid #: SW 1735
# Lots:	# Tracts: 3	Total # parcels: 3

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Signature	<input type="checkbox"/> Owner <input checked="" type="checkbox"/> Representative (Representatives must provide written proof of authorization)	Date
		8/14/19
Print Name		
Dave Hale		

Accepted by:	Poster & Affidavit:	Fee:	Case Number:	Requested Meeting Date:
Karrie Gredig	N/A	\$3,775	SI2514	P.O. 10/11/19

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POTABLE WATER AND WASTE WATER DISPOSAL

Potable Water provide by: ☒ Public utility ☐ Community well ☐ Private well
 Wastewater disposal method: ☒ Public utility ☐ Community system ☐ Private on-site

APPLICATION REQUIREMENTS

1 copy required: ☒ Signed application (original)
 ☒ Watershed sign off form, completed
 ☒ 8½" by 11" reduced copy of plat
 ☒ Certificate to Plat
 4 copies required: ☒ Subdivision drainage plan
 9 copies required: ☒ Topographic map of platted area on preliminary plat
 45 copies required: ☒ Signed application (copies)
 (35 copies for a ☒ Preliminary plat
 short plat) ☒ Summary of community meeting(s) (not required for short plat)
 (Additional information may be required)

Additional required documents unless specifically waived by Platting Officer:

☒ Soils investigation and analysis reports (4 copies) Waived by _____

Southcentral
Foundation



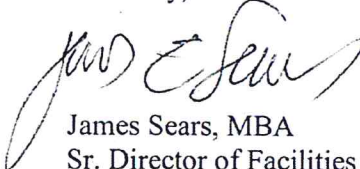
May 23, 2019

RE: Southcentral Foundation 4330 Elmore Property

To Whom It May Concern:

This Letter Authorizes R&M Consultants, Inc. to represent Southcentral Foundation on 4330 Elmore Road properties for Municipal Planning, Permitting and Platting activities. If you have questions or concerns, please contact me at 907-729-5261 or at jsears@SouthcentralFoundation.com.

Sincerely,


James Sears, MBA
Sr. Director of Facilities

WMS WATERCOURSE MAPPING SUMMARY

Per the requirements for watercourse verification outlined in Project Management and Engineering Operating Policy and Procedure #8 and Planning Department Operating Policy and Procedure #1 (effective June 18, 2007), MOA Watershed Management Services has inspected the following location for the presence or absence of stream channels or other watercourses, as defined in Anchorage Municipal Code (21.35).

- Project Case Number or Subdivision Name: Athenian Village Subd
- Project Location, Tax ID, or Legal Description: Tracts G5 & G6 (Plat 83-62) and Tract G4-1 (Plat 2007-114).
- Project Area (if different from the entire parcel or subdivision): Tudor & Elmore, Anchorage South Central Foundation lots

In accordance with the requirements and methods identified, WMS verifies that this parcel, project area, or application:

X ~~K~~

DOES NOT contain stream channels and/or drainageways, as identified in WMS field or archival mapping information.*

_____ **DOES** contain stream channels and/or drainageways **AND** these are located and identified on submittal documents in general congruence with WMS field and archival mapping information.

*New or additional mapping **IS NOT REQUIRED**.**

_____ Contains stream channels and/or drainageways **BUT** one or more streams or other watercourses:

- are **NOT** shown on submittal documents, or
- are **NOT** depicted adequately on submittal documents for verification, or
- are **NOT** located or identified on submittal documents in general congruence with WMS field and archival mapping information.

*New or additional mapping **IS REQUIRED** and must be re-submitted for further review and verification.**

_____ Presence of stream channels and/or drainageways is unknown **AND** field verification is not possible at this time. WMS will verify as soon as conditions and prioritized resources allow.


* *Streams omitted in error by WMS or others remain subject to MOA Code and must be shown in new mapping upon identification of the error.*

ADDITIONAL INFORMATION:

☐ Y ☒ N WMS written drainage recommendations are available. ☐ Preliminary ☐ Final
☐ Y ☒ N WMS written field inspection report or map is available. ☐ Preliminary ☐ Final
☐ Y ☒ N Field flagging and/or map-grade GPS data is available.

Inspection Certified By:

Date:



5/22/19



Mayor
Ethan Berkowitz

Anchorage Water & Wastewater Utility



Board Chair
Aaron D. Dotson

AWWU REQUIRED INFORMATION FOR PRE-PLATTING

- Project Case Number or Subdivision Name: Athenian Village S12514
- Project Location, Tax ID, or Legal Description: 008-024-05, 008-024-06, 008-024-09

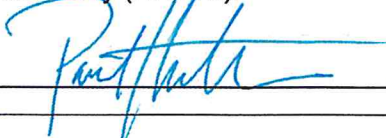
- Is this parcel located within AWWU's certificated service area? ----- ~~X~~ / N
- Is a water key box located on each parcel? ----- Y / ~~N~~
 - Does this service meet DCPM Standard? ----- ~~X~~ / N
- Is sewer stubbed to each parcel? ----- Y / ~~N~~
 - Does this service meet DCPM Standard? ----- ~~X~~ / N
- Are there any water or sewer connections that require removal? ----- ~~X~~ / N
- Are there any additional easements needed? ----- Y / ~~N~~
- Have any Private System plans been submitted for review? ----- Y / ~~N~~
- Are any of the lots subject to extended connection or other agreements? ----- Y / ~~N~~
- Does this platting action consolidate a previously connected (on-property) parcel with an unassessed parcel? ----- ~~X~~ / ~~N~~

If the parcel or subdivision is within an assessment area, please populate the table below with the relevant information (as balances may change year to year, this table represents a point in time as specified in the column "Year").

	Levied	Assessment Balance	Year
Water Lateral	X / N		
Water Transmission	Y / N		
Sewer Lateral	X / N		
Sewer Trunk	X / N		

- Comments:
Service on existing Tract G4-1 will require disconnection. Elimination of the Sanitary Sewer main to eliminate the easement can be handled with Private System drawings along with the water.

Verified By (AWWU):



Date:

8/19/19

Anchorage Water & Wastewater Utility  Clearly

3000 Arctic Boulevard • Anchorage, Alaska 99503
Phone 907-564-2774 • Fax 907-562-0824 • www.awwubiz



August 12, 2019

R&M No. 2731.01



R&M CONSULTANTS, INC.

9101 Vanguard Drive
Anchorage, Alaska 99507

Phone: 907.522.1707
Fax: 907.522.3403

Municipality of Anchorage
Planning Department
4700 Elmore Road
Anchorage, AK. 99507

RE: Preliminary Plat for Proposed Tracts G5 & G6 (Plat 83-62) and Tract G4-1 (Plat 2007-114), Athenian Village Subdivision

Dear Sir or Madam:

The attached preliminary plat affects the following existing legal descriptions:

Tracts G5 and G6, Athenian Village Subdivision, Plat No. 83-62

Tract G4-1, Athenian Village Subdivision, Plat No. 2007-114

All three tracts are owned by the Southcentral Foundation (SCF). The intention of this platting effort is to replat the three tracts into a single large tract and vacate the 20' Telephone, Electric and Sanitary Sewer Easement shown running north-south within the properties.

The purpose of the replat is because the SCF would like to demolish the existing building shown within existing Tract G5, and build a new facility that sits near the north edge of the proposed new tract and straddles the lot line abutting Tracts G4-1 and Tract 6.

Tracts G5 and G6 are zoned PLI, however Tract G4-1 is zoned B-3. There is a re-zone request being reviewed concurrently with the platting action to change the zoning of Tract G4-1 into PLI.

During construction of the new building, all utilities that fall within the proposed 20' easement vacation will be relocated to fall within existing easements that run along the north boundary.

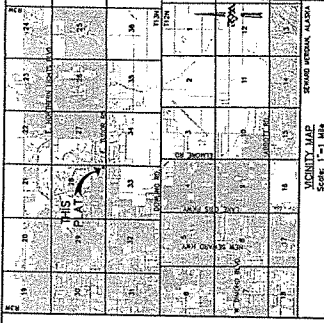
There is also a proposed access easement that is being facilitated between SCF and UAA that will potentially allow access to the new tract via E. 43rd Place.

Thank you for considering this plat request. Please forward any questions or comments to Dave Hale, R&M Consultants, at 646-9651.

Sincerely,

David C. Hale, PLS

R&M Consultants, Inc.



LEGEND

— DRAINAGE ARROW

SURVEYOR'S CERTIFICATE

DAVID C. HALE, PROFESSIONAL LAND SURVEYOR, PREPARED THIS MAP FOR THE PURPOSES OF RECORDING AND FOR THE PURPOSES OF THE ALASKA DEPARTMENT OF REVENUE AND TAXATION. THE MAP WAS PREPARED BY ME OR UNDER MY CLOSE PERSONAL SUPERVISION AND TO THE BEST OF MY KNOWLEDGE AND BELIEF IT ACCURATELY REPRESENTS THE FACTS AND CONDITIONS AS SHOWN AND LOCAL DRAINAGE PATTERNS MAY DIFFER FROM THE MORE GENERALIZED PATTERN SHOWN.

DATE 08/21/9
 SURVEYOR'S SIGNATURE
 DAVID C. HALE, L.S. 70335



ATHENIAN VILLAGE SUBDIVISION
 TRACT G5-1
 WITH VACATION OF 20' TELEPHONE,
 ELECTRIC, AND SANITARY SEWER EASEMENT
 A MORE REVISION OF TRACTS G2 AND G3
 (PLAN NO. 2007-114), ATHENIAN VILLAGE SUBDIVISION,
 LOCATED WITHIN THE SE 1/4, SECTION 28, TOWNSHIP 13 NORTH,
 RANGE 14 EAST, ALASKA
 ANCHORAGE RECORDING DISTRICT

9101 Vantage Drive, Anchorage, Alaska 99507
 PH (907) 522-1307 FAX (907) 522-3403
 www.mmcconsult.com
 AEOC 111

DATE 02-19-15
 SHEET 1 OF 1
 ORDER 2375.16
 PROJECT 2375.16
 DRAWN BY 2476



Memorandum

To: Ed Zernia

From: Bob Pintner, P.E.

Subject: Preliminary Geotechnical Recommendations
SCF - 4330 Elmore Road, Anchorage, Alaska

Date: May 7, 2019

Project #: 2731.01

R&M has completed a preliminary geotechnical investigation for the proposed new building at 4330 Elmore Road, Anchorage, Alaska. It is understood that a multi-story building with below grade parking is planned.

This geotechnical investigation is intended to be preliminary in nature. Additional design level geotechnical investigation will be necessary once the alternative site layout has been selected.

Two boreholes were drilled on April 17, 2019. Approximate borehole locations are shown on the attached Drawing 1. The borehole logs are included as Drawings 2-4, General Notes, and Explanation of Selected Symbols are shown on Drawing 5 and 6, respectively. Test holes were drilled using a Geoprobe 7620 DT drill rig owned and operated by Discovery Drilling, Inc. The borings were advanced using a continuous flight, 8-inch hollow-stem auger. Samples were obtained using a 1.4-inch I.D. (2.0-inch O.D.) sampler and a 140-pound automatic hammer (Standard Penetration Test). The penetration resistance, defined as the number of blows required to drive the sampler the last 12 inches of an 18-inch interval, gives an indication of the in-place relative density for unfrozen cohesionless soils. Blow counts per six-inch interval are presented on the boring logs.

Soil Conditions: Three general soil units were observed at the site: 1) a surficial fine-grained soil; 2) an alluvial gravel and sand deposit, and; 3) a fine-grained marine glacial deposit.

Surficial Fine-Grained Soil (overburden): A layer of organic debris and fine-grained soil was encountered in both test holes. This layer was 2.0 to 2.5 feet thick, and consisted of a few inches of organic debris grading to silty sand, containing roots and scattered organic debris.

Alluvial Gravel and Sand: This soil unit was encountered underlying the surficial soils. The unit extended to 23 feet in RM19-02, and extended below the bottom of RM19-01 (12'). The material consisted of well to poorly graded gravel with sand, with a few layers of poorly graded sand, and well graded sand with gravel and silt. The presence of cobbles and boulders is expected based on observations of drill rig reaction, and sampler refusal. The material was interpreted to be medium dense to very dense.

Marine Glacial Silt and Clay: This fine-grained soil unit underlies the alluvial material. The material consisted of sandy silt, silt with sand, sandy silty clay, and silty clay with sand. It was interpreted to be stiff to very stiff.

Groundwater Conditions: Groundwater was encountered in both test holes. In RM19-01 the groundwater level after completion of drilling was about 7.2 feet below the ground surface (bgs), and in RM19-02 water was at 8.8 feet bgs. Some seasonal fluctuation should be expected. The groundwater was encountered in

Memo to: Ed Zernia
From: Bob Pintner, P.E.
Date: 5/7/2019
Page 2

the alluvial materials which have a high permeability. Excavations below groundwater are expected to fill with water rapidly. Dewatering will be necessary in excavations below about 7 to 8 feet.

Geotechnical Recommendations

Our preliminary geotechnical design recommendations regarding excavation requirements, site drainage, foundation design, and other construction related elements are presented in the following paragraphs. These recommendations are based on our understanding of the subsurface data obtained from the test borings, and of the proposed construction of the proposed new building. It is emphasized that our understanding of the planned facility is limited to only general information regarding design of structure and multiple alternatives for building location and site layout. Because the project design is still at an early stage, our recommendations are necessarily somewhat broad. Additional design level geotechnical investigation will be necessary to develop final geotechnical recommendations for the project.

Earthwork Materials: For the purpose of this project, the earthwork materials for filling and backfilling should conform to the Municipality of Anchorage specifications for Classified Fill or Backfill. The specific gradation (Type II, IIA, etc.) recommended below structures and pavements are specified in the corresponding sections below. Materials excavated on-site may be used as fill or backfill if it meets the above criteria, is stockpiled separately, and is kept free of organics, other debris, and excess moisture.

Excavations: All excavation (e.g. for foundation, utility trenches, etc.) slopes should conform to Federal and State standards as a function of the depth, exposed soil type, moisture/groundwater condition, time left open, and adjacent surface loads, foundations or traffic. The site soils generally classify as Type C (29 CFR Parts 1926.650 - 652). Excavated cuts may not be stable at slopes steeper than about 1.5 (H) to 1 (V), especially when exposed to groundwater seepage, or surface water flows.

Groundwater should be expected in trenches and excavations extended deeper than about 7 to 8 feet bgs as may need to be excavated for building construction or utilities. Excavations which encounter groundwater will be unstable and the need for dewatering (and shields in trenches) should be expected. Use of sump pits and pumping procedures within some of the excavations should be anticipated. Surface water should also be controlled by grading the surface to drain away from excavations.

Foundations: Based on our preliminary geotechnical investigation, the use of conventional shallow foundations and slab-on-grade floors is suitable at this site. For foundations bearing in soil that is normally maintained above freezing, the base of perimeter footings should be buried at least 42 inches below final exterior grade, with interior footings buried at least 12 inches below adjoining finish grade. Footings should be a minimum of 16 inches in width. It is recommended that at least two inches of board insulation be placed on the exterior surface of perimeter foundation walls below grade (to top of footing) and on the interior surface of perimeter foundation walls for the portion that extends above grade. The insulation should be non-water absorbing, and approved for exterior and below ground use, considering subjection to a wet, cyclic freeze-thaw environment. Due to the high likelihood of groundwater seepage at the footing elevations, all exterior footings should be provided with perimeter foundation drains.

Earthwork for Foundations and Slabs: All building and retaining wall foundations should bear on undisturbed alluvial gravel/sand soils, or Type II fill over a prepared, stable subgrade. As a minimum, all fine-grained surficial soils should be completely removed and wasted from below the base of all footings. Further, the minimum excavation depth should continue as needed to completely remove any peat, organic soils, frozen soil, or debris if encountered. After excavation, the entire subgrade should be inspected and compacted to at least 95 percent of maximum laboratory unit weight (ASTM D 1557). Materials that cannot be re-compacted (e.g. too wet, very poorly-graded or contain a high fines content) should be over-excavated and replaced with classified backfill.

Allowable Bearing Pressures: Footings bearing on alluvial gravel and sand: The net allowable bearing pressure of shallow strip and spread footings with a minimum embedment of one foot and minimum width of 16 inches, bearing on a minimum of 3 feet of undisturbed alluvial gravel and sand, or compacted Type II material is 3,000 psf. The allowable capacity may be increased to 4,000 psf for footings with a minimum embedment of 1.5 feet, and a minimum width of 1.5 feet. The allowable pressures may be increased by one-third for seismic and wind loads. The allowable bearing pressure values include a factor of safety of 3.0.

Resistance to Lateral Loads: For resistance to lateral loads, the ultimate passive equivalent fluid pressure of 400 pcf may be used if the foundation or wall can move laterally at least 2 percent of the buried depth. One-half of the ultimate passive pressure may be used if only 0.5 percent movement can be tolerated. If even less movement is tolerable, the at-rest pressure of 60 pcf should be used. The coefficient of friction between the soil and concrete may be taken as 0.45. When combining friction and passive resistance, reduce the passive resistance by 50 percent. The values for passive resistance and friction do not include a factor of safety.

Concrete Floor Slabs: It is understood that the lower parking level of the facility will be constructed below grade. It is recommended that the depth below grade be limited to about 6 to 7 feet (referencing existing ground surface elevation at Test Boring RM19-02). This would place the floor about 2 feet above the highest observed groundwater elevation. The groundwater elevation is likely to fluctuate seasonally, and from year to year, depending on precipitation. It is possible that the groundwater could temporarily rise above the proposed floor elevation. At a minimum, a perimeter drain should be constructed around the lower level of the structure. Consideration should be given to waterproofing the walls and slabs.

Conventional slab-on-grade floors can be supported on the undisturbed native soils, or compacted fill material. We recommend that the slabs be underlain by a capillary break material, consisting of at least four inches of clean, free draining sand and gravel or crushed rock containing less than 3 percent fines passing the No. 200.

Retaining and Basement Walls: The following table summarizes the resultant lateral forces, locations and pressure distribution for design of earth retaining structures. The tabulated "retaining wall" parameters apply when the top of the structure is unrestrained and free to rotate away from the resultant "active" force on the order of 0.002H. The tabulated "basement wall" parameters apply when the top and base of the wall are restrained from moving.

	Resultant Lateral Force (lb/ln.ft)	Pressure Distribution	Resultant Above Base
Active Forces	PA = PS + PE + Pq		
Retaining Wall			
Static	PS = 15H ²	Triangle	H/3
Seismic	PE = 17H ²	Rectangle	H/2
Surcharge	Pq = 0.26qH	Rectangle	H/2
Basement Wall			
Static	PS = 28H ²	Triangle	H/3
Seismic	PE = 20H ²	Rectangle	H/2
Surcharge	Pq = 0.41qH	Rectangle	H/2
Passive Forces	PP = 200H ²	Triangle	H/3

These parameters are based on the assumption of walls being designed and constructed per the following conditions:

- H is the height (ft) of the wall measured from the ground surface to the base (behind the wall for active forces and in front of the wall for passive forces). The coefficient q is the surcharge (psf) load on the ground surface above the wall. The “passive forces” only apply to the retaining walls.
- The tabulated resultant forces assume: 1) The wall excavations are backfilled with cohesionless, well draining, non-frost susceptible materials conforming to MOA Type II material, 2) the backfill is level behind and in front of the wall, 3) the back face of the walls are vertical (no pitch), and neglect friction between the wall and backfill.
- The resultant “active forces” for seismic conditions assume an effective ground motion of 85 percent of the “peak ground acceleration” (used $PGA = 0.40g$). And;
- The external stability of all retaining walls should exhibit a minimum safety factor against sliding of at least 1.5 under static loading, and 1.1 under seismic loading. In order to fully mobilize the passive resistance, the wall footing must move laterally about two percent of the height that the passive pressure will be applied. One-half of the maximum passive resistance may be developed if the footing can move about 0.5 percent of its height. If this amount of horizontal movement cannot be tolerated, a reduced lateral resistance equal to the at-rest earth pressure condition should be used.

Seismic Ground Motions. The seismic design parameters shown below address the geotechnical aspects of designing foundations on this site relative to the International Building Code (IBC) (ICC, 2012)¹.

Seismic Design Parameters

Site Parameters	Reference	Value
Class	Table 1613.5.2	D
S_s	Figures 1613.5 (11) &(12)	1.25
S_1		0.55
F_a	Tables 1613.5.3 (1) & (2)	1.0
F_v		1.5

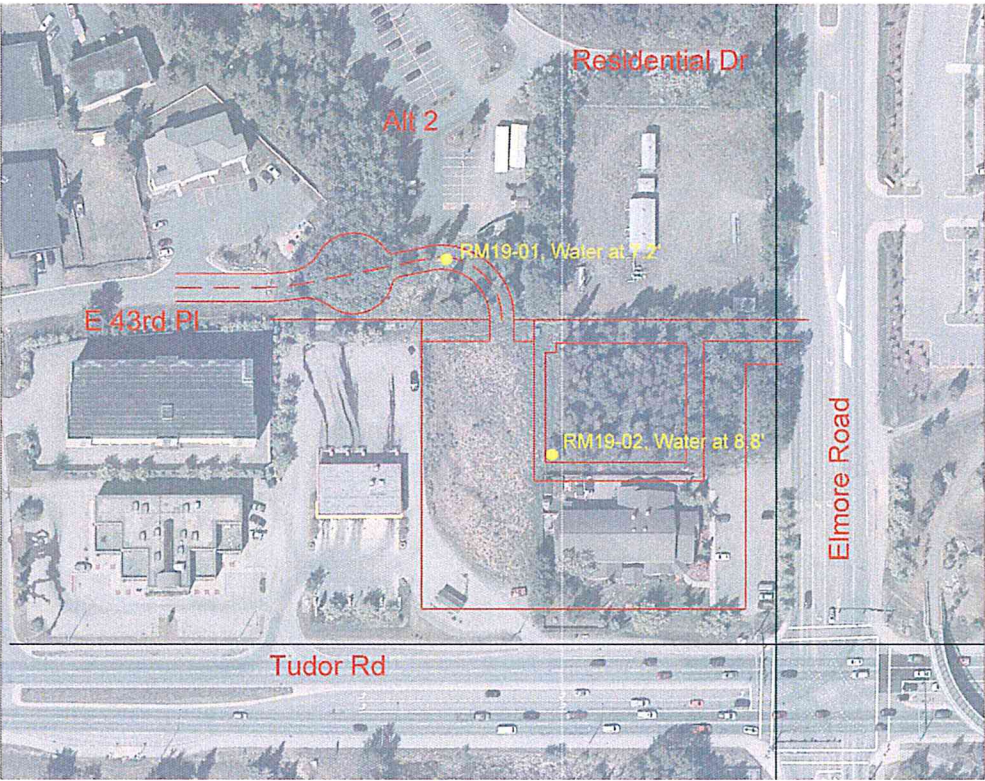
CLOSURE

It is emphasized that the information provided in this memorandum is preliminary in nature and additional design level geotechnical investigations should be performed to support design and construction of the project.

R&M Consultants, Inc. performed this work in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions. No warranty, express or implied, beyond exercise of reasonable care and professional diligence, is made. This report is intended for use only in accordance with the purposes of study described within.

¹ International Code Council, Inc. (ICC, 2012), “International Building Code” (IBC), 2012.

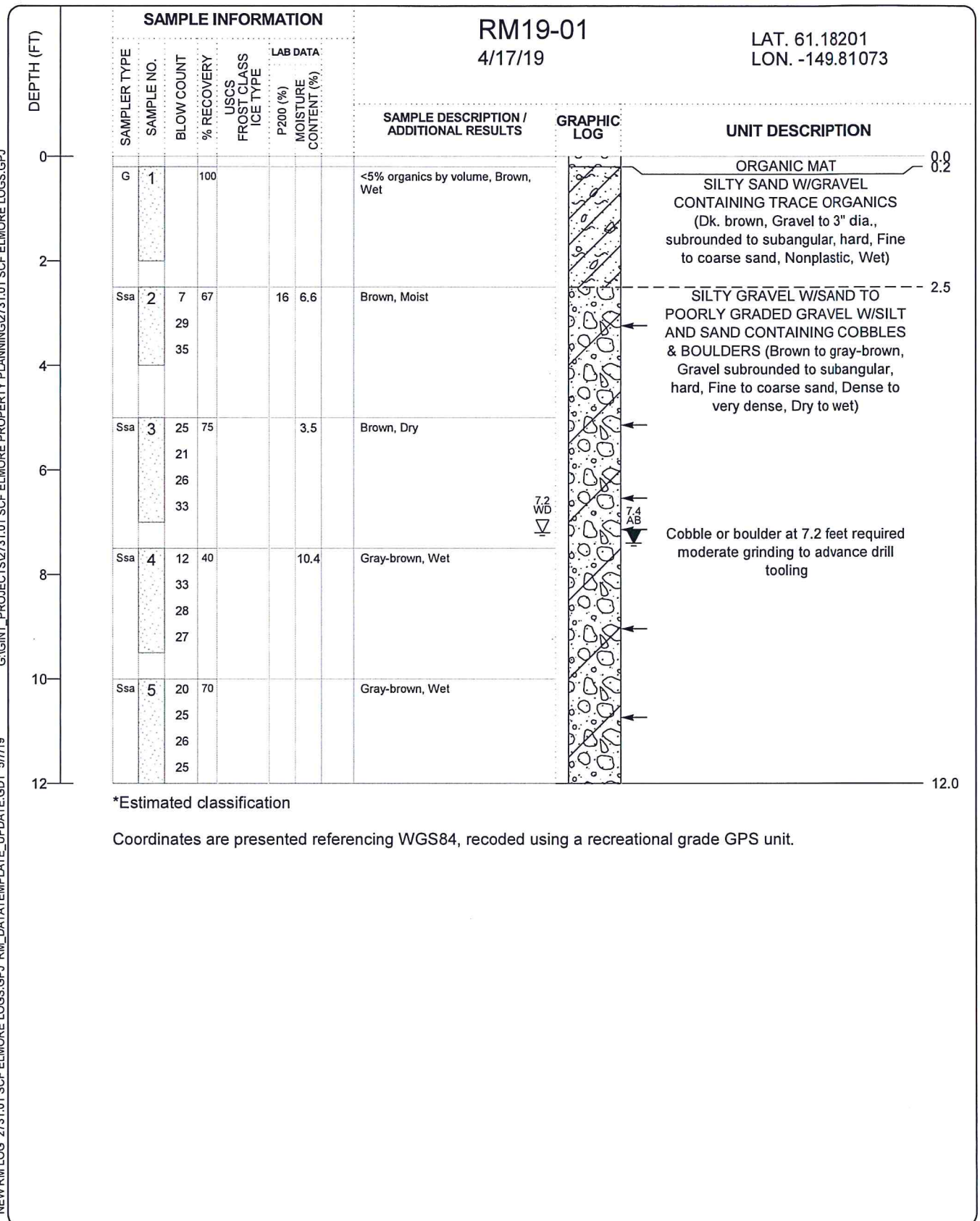
Memo to: Ed Zernia
From: Bob Pintner, P.E.
Date: 5/7/2019
Page 5



Drawing 1 – Borehole Location Map

G:\GINT_PROJECTS\2731.01 SCF ELMORE PROPERTY PLANNING\2731.01 SCF ELMORE LOGS.GPJ

NEW RM LOG 2731.01 SCF ELMORE LOGS.GPJ RM_DATATEMPLATE_UPDATE.GDT 5/7/19



DWN:	A.T.B.
CKD:	R.M.P.
DATE:	MAY '19
SCALE:	SHOWN

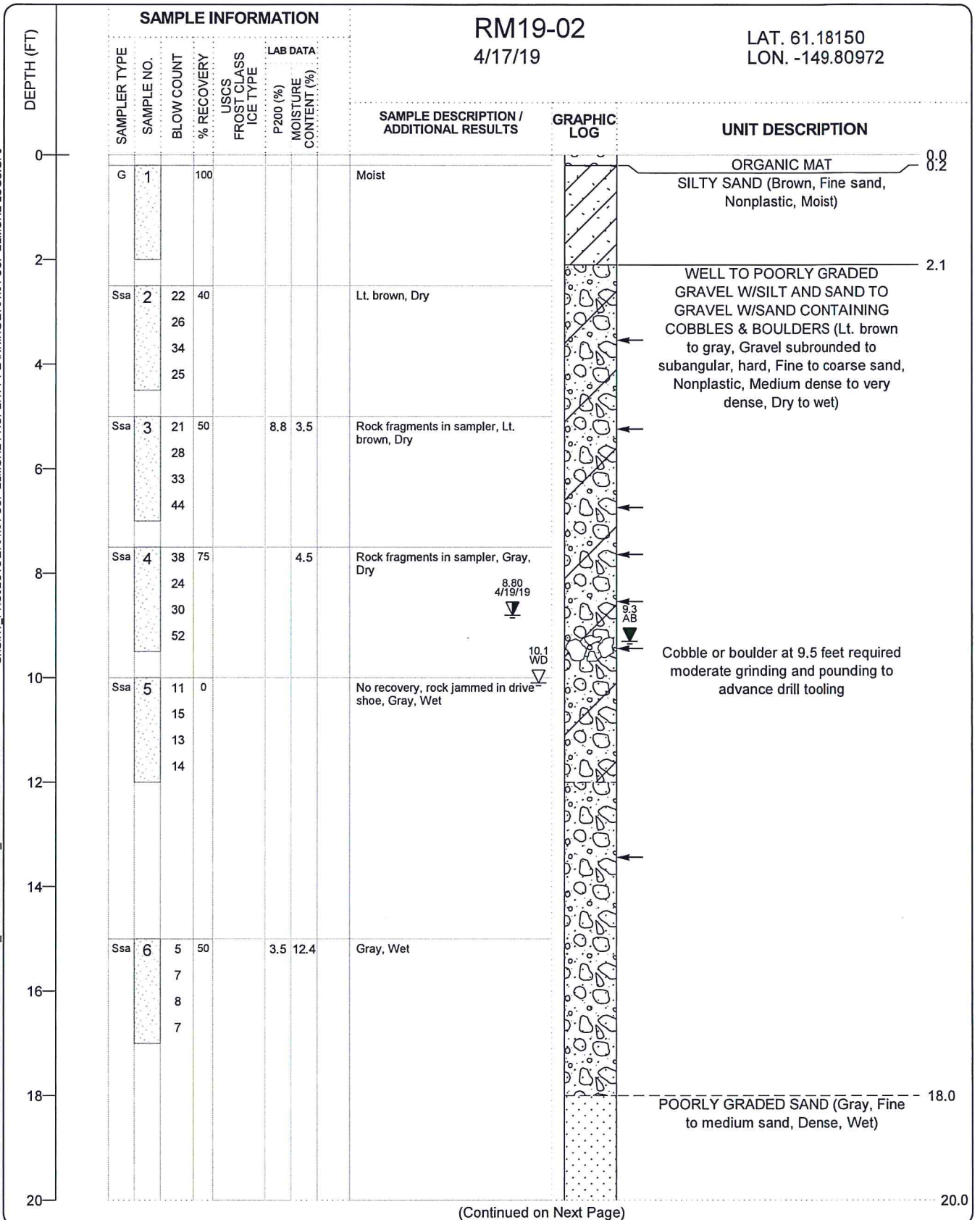


SCF ELMORE PROPERTY PLANNING
ANCHORAGE, ALASKA
LOG OF TEST BORING
RM19-01

FB:	NA
GRID:	1735
PROJ.NO:	2731.01
DWG.NO:	2

G:\GINT_PROJECTS\2731.01 SCF ELMORE PROPERTY PLANNING\2731.01 SCF ELMORE LOGS.GPJ

NEW RM LOG 2731.01 SCF ELMORE LOGS.GPJ RM_DATATEMPLATE_UPDATE.GDT 5/7/19



DWN: A.T.B.
 CKD: R.M.P.
 DATE: MAY '19
 SCALE: SHOWN



SCF ELMORE PROPERTY PLANNING
 ANCHORAGE, ALASKA
 LOG OF TEST BORING
 RM19-02

FB: NA
 GRID: 1735
 PROJ.NO: 2731.01
 DWG.NO: 3

DEPTH (FT)	SAMPLE INFORMATION						RM19-02 (CONTINUED)		
	SAMPLER TYPE	SAMPLE NO.	BLOW COUNT	% RECOVERY	USCS FROST CLASS ICE TYPE	LAB DATA P200 (%) MOISTURE CONTENT (%)	SAMPLE DESCRIPTION / ADDITIONAL RESULTS	GRAPHIC LOG	UNIT DESCRIPTION
20	Ssa	7	9	67		22.4	1.5 feet of heave which was able to be cleared, Gray, Wet		POORLY GRADED SAND (Gray, Fine to medium sand, Dense, Wet)
22			15						
24			18						
26	Ssa	8	16	30			1/8 inch thread was able to be rolled, Gray, Wet		SILTY-CLAYEY SAND W/GRAVEL (Gray, Gravel to 0.5" dia., subrounded to subangular, hard, Fine to medium sand, Low plasticity, Medium dense, Wet)
28			16						
30	Ssa	9	24	65		21.2	Gray, Wet		SANDY SILT (Gray, Fine sand, Nonplastic, Dense, Wet)
32			16						
34			16						
36	Ssa	10	9	75			Gray, Wet		INTERLAYERED SANDY SILT AND POORLY GRADED SAND (Gray, Fine to medium sand, Nonplastic, Dense, Wet)
37.0			11						
			30						
			27						

*Estimated classification

Coordinates are presented referencing WGS84, recoded using a recreational grade GPS unit.

1-inch diameter PVC standpipe was installed to 35 feet below ground surface and screened from 7 to 17 feet.

DWN: A.T.B.
CKD: R.M.P.
DATE: MAY '19
SCALE: SHOWN



SCF ELMORE PROPERTY PLANNING
ANCHORAGE, ALASKA
LOG OF TEST BORING
RM19-02

FB: NA
GRID: 1735
PROJ.NO: 2731.01
DWG.NO: 4

SOILS CONSISTENCY AND SYMBOLS

CLASSIFICATION: Identification and classification of the soil is accomplished in accordance with the ASTM version of the Unified Soil Classification System. When laboratory testing data on material passing the 75-mm sieve is available Standard D 2487 (Classification of Soils for Engineering Purposes) is used and when laboratory data is not available D 2488 (Visual-Manual Procedure) is used. This classification system identifies three major soil divisions: coarse-grained soils, fine-grained soils, and highly organic soils. These three divisions are further subdivided into a total of 15 basic soils groups. Based on the results of visual observations and prescribed laboratory tests, a soil is catalogued according to the basic soil groups, assigned a group symbol(s) and name, and thereby classified. Flow charts contained in the two standards can be used to assign the appropriate group symbol(s) and name.

SOIL DENSITY/CONSISTENCY - CRITERIA: Soil density/consistency as defined below and determined by normal field and laboratory methods applies only to non-frozen material. For these materials, the influence of such factors as soil structure, i.e. fissure systems shrinkage cracks, slickensides, etc., must be taken into consideration in making any correlation with the consistency values listed below. In permafrost zones, the consistency and strength of frozen soil may vary significantly and inexplicably with ice content, thermal regime and soil type.

COARSE GRAINED (DOT&PF 2007)

<u>Relative Density</u>	<u>N * (blows/FT.)</u>
Very loose	0 - 4
Loose	5 - 10
Medium dense	11 - 30
Dense	31 - 50
Very dense	>50

FINE GRAINED (ASTM D2488)

<u>Consistency</u>	<u>Thumbail Test</u>
Very soft	Thumb > 1 in.
Soft	Thumb = 1 in.
Firm	Thumb = 1/4 in.
Hard	Thumbail indents
Very hard	Thumbail will not indent

* Standard Penetration "N": Blows per 12 inches of a 140-pound manual hammer (lifted with rope & cathead) falling 30 inches on a 2-inch O.D. split-spoon sampler except where noted. Blow counts presented on test boring logs are direct field values (i.e. they have not been corrected to account for hammer efficiency, borehole diameter, sampling method, or rod length)

KEY TO TEST RESULTS

DD - Dry Density	PP - Pocket Penetrometer
LL - Liquid Limit	P200 - % Passing No. 200 Screen
MC - Moisture Content	P.02 - % Passing 0.02 mm
Org - Organic Content	P.005 - % Passing 0.005 mm
PI - Plastic Index	P.002 - % Passing 0.002 mm
PL - Plastic Limit	

* (DRAWING 1 NEW SOIL CONSISTENCY&CLASS (NON-DOT&PF) 5/7/19 02:05 PM

DWN:	B.M.M.
CKD:	A.T.B.
DATE:	GENERAL
SCALE:	NONE



GENERAL NOTES

FB:	N/A
GRID:	N/A
PROJ.NO:	GENERAL
DWG.NO:	5

STANDARD SYMBOLS

SYMBOL	NAME	PARTICLE SIZE	SYMBOL	NAME
	CLAY	< 0.002mm, Plastic		ORGANICS
	SILT	0.002mm, - #200		ICE
	SAND	#200, - #4		ICE W/SOIL INCLUSIONS
	GRAVEL	#4, - 3"		ICE LENSE IN SOIL
	COBBLES & BOULDERS	3" - 12" & > 12"		ICE CRYSTALS IN CLAY

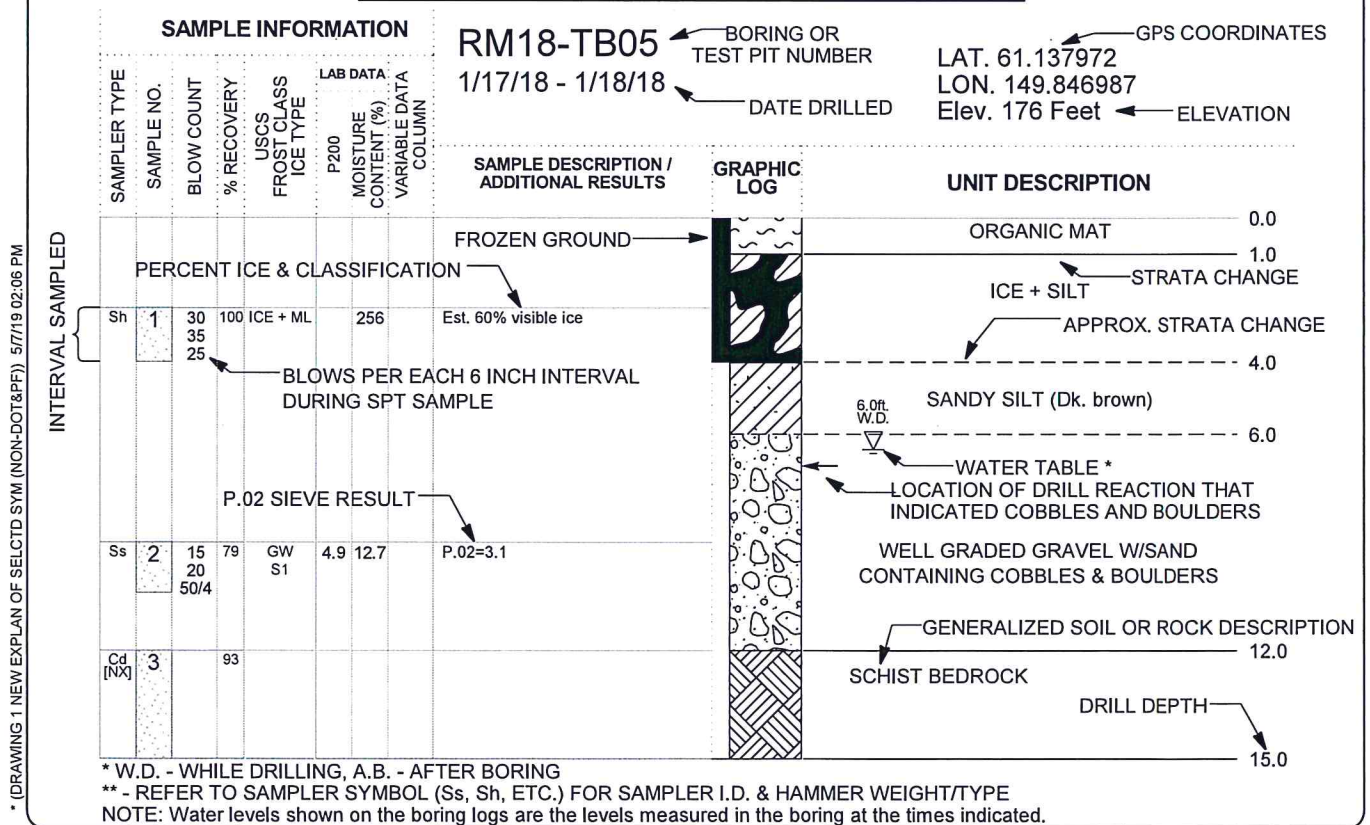
(The symbols shown above are frequently used in combinations, e. g. SILTY GRAVEL W/SAND)

SAMPLER TYPE SYMBOLS

A Auger Sample	MC 1.5 In. I.D. Macro-core	Ss 1.4 In. Split Spoon w/140 lb. Manual Hammer
C Cuttings Sample	MC7 3.0 In. I.D. Macro-core	Ssa 1.4 In. Split Spoon w/140 lb. Auto Hammer
Cd Double Tube Core Barrel	Sh 2.5 In. Split Spoon w/340 lb. Manual Hammer	Tm Modified Shelby Tube
Cs Single Tube or Auger Core	Sha 2.5 In. Split Spoon w/340 lb. Auto Hammer	Ts 3.0 In. Shelby Tube
Ct Triple Tube Core Barrel	SI 2.5 In. Split Spoon w/140 lb. Hammer	[XX] Sampler ID (Rock Core - NX, NQ, etc.)
G Grab Sample		

NOTE: Sampler types are either noted above the boring log or adjacent to it at the respective depth. An individual log may not utilize all of the items listed.

TYPICAL BORING AND TEST PIT LOG



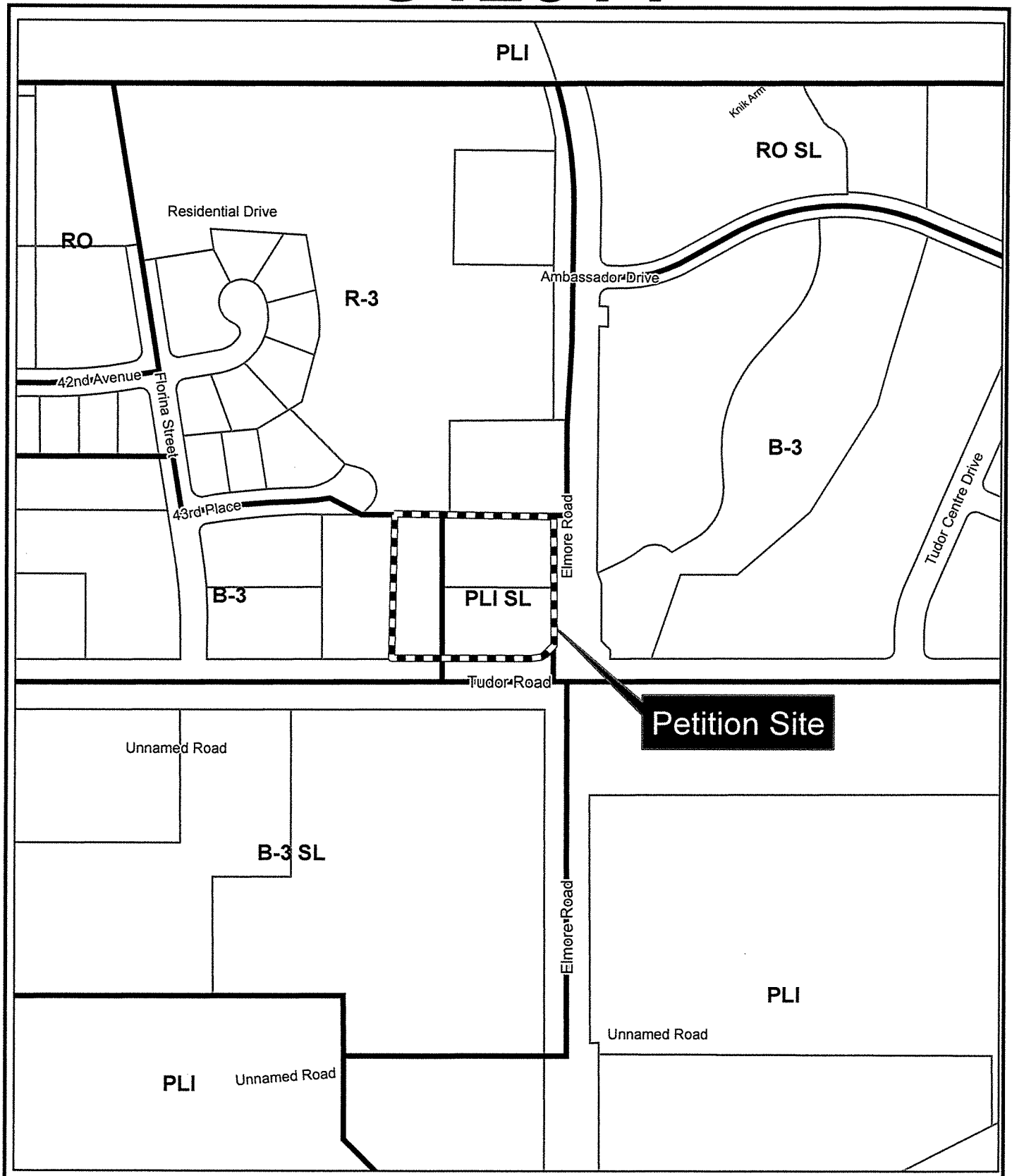
DWN: B.M.M.
 CKD: A.T.B.
 DATE: GENERAL
 SCALE: NONE



EXPLANATION OF SELECTED SYMBOLS

FB: N/A
 GRID: N/A
 PROJ.NO: GENERAL
 DWG.NO: 6

S12514



Municipality of Anchorage
Planning Department

Date: September 3, 2019

360 180 0 360 Feet

